

SPECIAL BOARD MEETING

Wednesday, September 28, 2022 4:30 PM

HS CONFERENCE ROOM

705 N 9th Street

Arlington, NE 68002

1. Special Meeting Called to Order and Roll Call
 - 1.1. Pledge of Allegiance
2. Discuss, Consider and Take Necessary Action to Adopt Property Tax Request Resolution for 2022-2023 Budget Year as Presented at Joint Agency Meeting

General Fund: \$7,332,763.00

Bond Fund: \$454,545.00

Special Building Fund: \$0

Qualified Capital Purpose Undertaking Fund: \$303,030.00

3. Discuss and Consider Change Order Billing from Boyd Jones/DLR for Helical Piers
4. Adjourn

RESOLUTION SETTING THE PROPERTY TAX REQUEST

RESOLUTION NO. _____

WHEREAS, Nebraska Revised Statute 77-1632 and 77-1633 provides that the Governing Body of Arlington Public Schools passes by a majority vote a resolution or ordinance setting the tax request; and

WHEREAS, a special public hearing was held as required by law to hear and consider comments concerning the property tax request;

NOW, THEREFORE, the Governing Body of Arlington Public Schools resolves that:

1. The 2022-2023 property tax request be set at:

General Fund:	\$	7,332,763.00
Bond Fund:	\$	454,545.00
Special Building Fund:	\$	-
Qualified Capital Purpose	\$	303,030.00
Undertaking Fund:		

2. The total assessed value of property differs from last year’s total assessed value by 4.06 percent.

3. The tax rate which would levy the same amount of property taxes as last year, when multiplied by the new total assessed value of property would be 0.956843 per \$100 of assessed value.

4. Arlington Public Schools proposes to adopt a property tax request that will cause its tax rate to be 1.049907 per \$100 of assessed value.

5. Based on the proposed property tax request and changes in other revenue, the total operating budget of Arlington Public Schools will increase (or decrease) last year’s budget by -43.72 percent.

6. A copy of this resolution be certified and forwarded to the County Clerk on or before October 15, 2022.

Motion by _____, seconded by _____ to adopt Resolution # _____.

Voting yes were:

Voting no were:

Dated this _____ day of _____, 2022



Boyd Jones Construction
950 South 10th Street, Suite 100
Omaha, Nebraska 68108
Phone: (402) 553-1804
Fax: (402) 561-7705

Project: 20-029 - Arlington Public Schools
705 North 9th Street
Arlington, Nebraska 68005

Prime Contract Change Order Request #001: Additional Helical Pier Depth

TO:	Arlington Public Schools 705 North 9th Street Arlington, Nebraska 68005	FROM:	Boyd Jones Construction Company 950 South 10th Street, Suite 100 Omaha, Nebraska 68108
CHANGE ORDER REQUEST NUMBER / REVISION:	001 / 0	PRIME CONTRACT CHANGE ORDER:	None
STATUS:	Approved	CREATED BY:	Michael Langner (Boyd Jones Construction Company)
SCHEDULE IMPACT:	0 days	DATE CREATED:	9/13/2022
EXECUTED:	No	SIGNED CHANGE ORDER RECEIVED DATE:	
		TOTAL AMOUNT:	\$24,396.96

CHANGE ORDER REQUEST TITLE: Additional Helical Pier Depth

CHANGE ORDER REQUEST DESCRIPTION:

Additional Pier Depth per ASI-4. Price based off unit rates established at bid time with Thrasher.

ATTACHMENTS:

POTENTIAL CHANGE ORDERS IN THIS CHANGE ORDER REQUEST:

PCO #	Contract Company	Title	Schedule Impact	Amount
1	Arlington Public Schools	Additional Helical Pier Depth	0 days	\$24,396.96
Total:				\$24,396.96

CHANGE ORDER REQUEST LINE ITEMS:

PCO # 1: Additional Helical Pier Depth

#	Budget Code	Description	Amount
1	3-330.S Pier Concrete.Sub Contracts	Additional Pier Depth	\$23,744.00
Subtotal:			\$23,744.00
BJC Fee (2.75% Applies to all line item types.):			\$652.96
Grand Total:			\$24,396.96

THRASHER™

COMMERCIAL

Thrasher, Inc.
11844 Valley Ridge Drive Papillion, NE 68046
Contact: Todd Royal
Cell: 402.490.0109
Phone: 800-827-0702 Fax: (402)393-4002
www.gothrashercommercial.com

SUBMITTED TO:

Boyd Jones Construction
950 South 10th St, Suite 100
Omaha, NE 68108

Mitch Broekemeier
Phone: 402.550.1786
Email: mbroekemeier@boydjones.biz

BID SUMMARY

Project Name: 2021031-Arlington Public Schools-Helical Piers

Project Location: 705 N 9th St Arlington, NE 68002

Bid Date: March 11, 2021

BID AMOUNT**\$50,000.00****SCOPE OF WORK**

This bid submittal includes all labor, materials, equipment and site supervision required to install (21) Retrofit Helical Piers and (6) New Construction Helical Piles as specified for the above referenced project. Proposal is based on a structural plan set, specification section 315001-foundation support, and a geotechnical report prepared by Olsson, dated July 9, 2020. Proposal includes all fees associated to provide submittals indicating all pier/pile information, pier/pile layout and capacities signed and sealed by a registered engineer in the state of Nebraska. Proposal may be subject to change pending approved stamped submittals.

PRODUCTS**(12) HP288 Helical Piers (Galvanized)**

- (12) Underpinning Bracket
- (12) HP288 7' Lead - 10"-12"-14"
- (48) HP288 7' Extension

(9) HP288 Helical Piers (Galvanized)

- (9) Underpinning Bracket
- (9) HP288 7' Lead - 10"-12"-14"
- (36) HP288 7' Extension

(6) HP287 Helical Piles (Galvanized)

- (6) 6"x6" New Construction Bracket w/ Coupling Hardware
- (6) HP287 7' Lead - 10"-12"-14"
- (24) HP287 7' Extension

SUBMITTED TO:Boyd Jones Construction
950 South 10th St, Suite 100
Omaha, NE 68108Mitch Broekemeier
Phone: 402.550.1786
Email: mbroekemeier@boydjones.biz**INSTALLATION****HP288 Helical Piers (Galvanized)**

- (12) HP288 Retrofit Helical Piers shall be installed along portions of the southern wall in area 'A' as shown on structural drawing S1.1. Thrasher, Inc. will excavate to bottom of foundation, clean and prepare footing and install retrofit foundation brackets. Each helical pier will be installed with a hydraulic-powered, rotary-torque drive unit. Shaft extensions will be added to reach design torque/depth and coupled with manufacturers supplied hardware.
- Each pier shall support a minimum design working load of 27 kips and be installed to an ultimate capacity of 54 kips (FOS 2.0) - axial compression loads only.
- Estimated bearing depths of (30) feet below B.O.F elevation as specified. Minimum final installation torque of at least 6,000 ft-lb.
- Monitor and document installation torque for each pier and provide data, including correlation of torque to capacity, to the client.
- After all of the piers are installed, uniformly load all of the piers to the design working load until the footing mobilizes or as instructed by the project engineer. Lock system in place.

HP288 Helical Piers (Galvanized)

- (9) HP288 Retrofit Helical Piers shall be installed along portions of the southern wall in area 'A' as shown on structural drawing S1.1. Thrasher, Inc. will excavate to bottom of foundation, clean and prepare footing and install retrofit foundation brackets. Each helical pier will be installed with a hydraulic-powered, rotary-torque drive unit. Shaft extensions will be added to reach design torque/depth and coupled with manufacturers supplied hardware.
- Each pier shall support a minimum design working load of 18 kips and be installed to an ultimate capacity of 36 kips (FOS 2.0) - axial compression loads only.
- Estimated bearing depths of (30) feet below B.O.F elevation as specified. Minimum final installation torque of at least 4,000 ft-lb.
- Monitor and document installation torque for each pier and provide data, including correlation of torque to capacity, to the client.
- After all of the piers are installed, uniformly load all of the piers to the design working load until the footing mobilizes or as instructed by the project engineer. Lock system in place.

HP287 Helical Piles (Galvanized)

- (6) HP287 New Construction Helical Piles will be installed with a hydraulic-powered, rotary-torque drive unit. Shaft extensions will be added to reach design torque/depth and coupled with manufacturers supplied hardware.
- Each pile shall support a minimum design working load of 18 kips and be installed to an ultimate capacity of 36 kips (FOS 2.0) - axial compression loads only.
- Estimated bearing depths of (30) feet below B.O.F elevation as specified.
- Minimum final installation torque of at least 4,000 ft-lb.
- Monitor and document installation torque for each pile and provide data, including correlation of torque to capacity, to the client.



Thrasher, Inc.
 11844 Valley Ridge Drive Papillion, NE 68046
 Contact: Todd Royal
 Cell: 402.490.0109
 Phone: 800-827-0702 Fax: (402)393-4002
 www.gothrashercommercial.com

SUBMITTED TO:

Boyd Jones Construction
 950 South 10th St, Suite 100
 Omaha, NE 68108

Mitch Broekemeier
 Phone: 402.550.1786
 Email: mbroekemeier@boydjones.biz

QUALIFICATIONS

HP288 Helical Piers (Galvanized)

- Thrasher, Inc. will excavate to bottom of foundation, prepare footing and install retrofit foundation brackets at (12) HP288 helical pier locations.
- An additional charge of \$210 per 7-foot section will be added if HP288 helical piers must be installed to a depth more than (30) feet in order to reach bearing strata.
- If any HP288 helical piers are needed beyond what is stated in this proposal or shown on the drawings, an additional charge of \$1,700 will be added per pier, for a depth up to (30) feet. Additional piers will only be installed following written client approval.

HP288 Helical Piers (Galvanized)

- Thrasher, Inc. will excavate to bottom of foundation, prepare footing and install retrofit foundation brackets at (9) HP288 helical pier locations.
- An additional charge of \$210 per 7-foot section will be added if HP288 helical piers must be installed to a depth more than (30) feet in order to reach bearing strata.
- If any HP288 helical piers are needed beyond what is stated in this proposal or shown on the drawings, an additional charge of \$1,700 will be added per pier, for a depth up to (30) feet. Additional piers will only be installed following written client approval.

HP287 Helical Piles (Galvanized)

- The general contractor is responsible for all excavating of footings or grade beams at (6) HP287 new construction locations. This shall be completed prior to the mobilization of Thrasher, Inc.'s equipment and materials to the site.
- The general contractor is responsible for marking the (6) new construction pile locations and establishing a benchmark for setting top of pile elevations.
- An additional charge of \$196 per 7-foot section will be added if HP287 helical piles must be installed to a depth more than (30) feet in order to reach bearing strata.
- If any HP287 helical piles are needed beyond what is stated in this proposal or shown on the drawings, an additional charge of \$1,400 will be added per pile, for a depth up to (30) feet. Additional piles will only be installed following written client approval.

Other Qualifications

- The general contractor is responsible for providing proper access for Thrasher, Inc.'s installation equipment.
- If during pier/pile installation, subsurface conditions of a type and location are encountered of a frequency that were not reported, inferred and/or expected at the time of preparation of the bid, the additional costs required to overcome such conditions shall be considered as extras to be paid for.
- This bid includes up to 1 mobilization to the work site. Additional client directed mobilizations will cost an additional \$3,000 per trip.
- All fees associated to provide submittals indicating all pier/pile information, pier/pile layout and capacities signed and sealed by a registered engineer in the state of Nebraska have been added to this proposal.
- This project is estimated to take approximately 6 days of contiguous work and weekend work would need to be pre-scheduled and at an additional cost. Lead time for crew & product is approximately four weeks after receipt of signed proposal/contract.
- If a full-scale test pile/load test is needed to confirm the capacity of the proposed helical piles, please add \$10,000 to this proposal.
- Proposal is valid for thirty (30) days.

EXCLUSIONS

- Damage to underground utilities or mechanical and electrical ductwork/conduits.
- Marking the (6) new construction helical pile locations and establishing a benchmark for top of pile elevations.
- Additional insurance coverage beyond Thrasher, Inc.'s standard coverage.
- Bonding. If required, please add 1.5% to contract amount.
- Structural or cosmetic damages due to the installation process.
- Prevailing wages.
- Providing traffic control services (if applicable).

BID AMOUNT
\$50,000.00

Thrasher, Inc.

SIGNATURE:

DATE: March 09, 2021

SIGNATURE: _____

DATE: _____

Acceptance of Proposal - The prices proposed, specifications and conditions are satisfactory and are hereby accepted. You are authorized to do the work as specified. We jointly and severally agree to pay you upon completion of the job, and will further pay your service charge of 1-1/3% per month (16% annum) if our account is 30 or more days past due, and your attorney's fees and costs to collect or enforce this contract. **My signature indicates that I accept the terms of this Proposal.



AIA® Document G710™ – 2017

Architect's Supplemental Instructions

PROJECT: *(name and address)*
10-19116-00_
Arlington Public Schools Addition and
Renovations
705 North 9th Street
Arlington, NE 68002

CONTRACT INFORMATION:
Contract For: General Construction

Date: September 2, 2020

ASI INFORMATION:
ASI Number: 004

Date: July 21, 2021

OWNER: *(name and address)*
Arlington Public Schools
705 N 9th
Box 580
Arlington, NE 68002

ARCHITECT: *(name and address)*
DLR Group inc. (a Nebraska corporation)
6457 Frances Street, Suite 200
Omaha, NE 68106

CONTRACTOR: *(name and address)*
Boyd Jones Construction
4360 Nicholas Street
Omaha, NE 68131

The Contractor shall carry out the Work in accordance with the following supplemental instructions without change in Contract Sum or Contract Time. Proceeding with the Work in accordance with these instructions indicates your acknowledgment that there will be no change in the Contract Sum or Contract Time.
(Insert a detailed description of the Architect's supplemental instructions and, if applicable, attach or reference specific exhibits.)

DESCRIPTION: Amend the Drawings to the above referenced project as follows:

DRAWINGS:

ITEM NO. 1. DRAWING S0.1 – STRUCTURAL NOTES

- A. Delete Drawing S0.1 in its entirety and substitute new Drawing as shown on Attachment No. S0.1.

ISSUED BY THE ARCHITECT:

DLR Group inc. (a Nebraska corporation)
ARCHITECT *(Firm name)*

SIGNATURE 

Steve Burgess
PRINTED NAME AND TITLE

July 21, 2021
DATE

STRUCTURAL NOTES

CODE: INTERNATIONAL BUILDING CODE, 2018 EDITION.

GENERAL NOTES: THE DRAWINGS REPRESENT THE FINISHED STRUCTURE, NOT THE METHOD OF CONSTRUCTION...

CONTRACTOR IS TO ESTABLISH AND VERIFY OPENINGS AND INSERTS FOR ITEMS TO BE INSTALLED BY OTHER TRADES PRIOR TO SUBMITTAL OF SHOP DRAWINGS AND CONSTRUCTION.

CONSTRUCTION MATERIAL AND EQUIPMENT PLACED ON FRAMED CONSTRUCTIONS SHALL BE SUCH THAT THE LOAD DOES NOT EXCEED THE DESIGN LIVE LOAD OF THE CONSTRUCTION...

DETAILS THAT ARE NOTED AS 'TYP.' OR 'ON DETAIL' TITLES ARE TO BE APPLIED TO THE PROJECT CONSTRUCTION AS GENERAL CONSTRUCTION METHODS UNLESS NOTED OTHERWISE...

WHERE DISCREPANCIES OCCUR BETWEEN GENERAL NOTES, PLANS, DETAILS, AND SPECIFICATIONS, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN, UNLESS VERIFIED OTHERWISE BY THE ARCHITECT AND ENGINEER IN WRITING.

THESE DOCUMENTS SHALL NOT BE CONSTRUED AS STAND-ALONE DOCUMENTS. CONTRACTOR SHALL COORDINATE WITH ALL OTHER CONSULTANTS WORK.

DO NOT SCALE DRAWINGS. CONSTRUCTION DOCUMENTS SHALL NOT BE REPRODUCED FOR USE OF SHOP DRAWINGS SUBMITTALS OR ANY OTHER PROJECT WITHOUT WRITTEN CONSENT BY DLR GROUP.

SEE ARCHITECTURAL DRAWINGS FOR COMPLETE STRUCTURAL DIMENSIONS.

SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON THE STRUCTURAL DRAWINGS. THIS INCLUDES, BUT IS NOT LIMITED TO, STEEL AT ROOF DRAINS, EMBEDDED PLATES, AND STEEL ASSOCIATED WITH ARCHITECTURAL DETAILS.

DESIGN DEAD LOADS INCLUDING SELF-WEIGHT: ROOF (TYPICAL): 25 PSF. ROOF (IF HC WITH 4" TOPPING): 135 PSF.

DESIGN LIVE LOADS: ROOF LIVE: 20 PSF. ROOF: SNOW LOADS IN ACCORD WITH INTERNATIONAL BUILDING CODE SECTION 1608 AND CHAPTER 7 OF ASCE 7...

CONCRETE CONTAINING SUPERPLASTICIZING ADMIXTURE SHALL HAVE A SLUMP NOT EXCEEDING 3" PRIOR TO ADDING ADMIXTURE AND NOT EXCEEDING 8" AT PLACEMENT.

THE ADDITION OF WATER TO A CONCRETE BATCH WITH INSUFFICIENT SLUMP SHALL NOT BE PERMITTED.

ULTIMATE DESIGN WIND SPEED, Vult = 120 MPH. NOMINAL DESIGN WIND SPEED, Vnom = 93 MPH.

WIND LOAD: ULTIMATE DESIGN WIND SPEED, Vult = 120 MPH. NOMINAL DESIGN WIND SPEED, Vnom = 93 MPH.

WIND COMPONENTS AND CLADDING PER ASCE 7-10: WALL PRESSURE (PSF), AREA, NEGATIVE ZONE 4, NEGATIVE ZONE 5, POSITIVE ZONE 4 & 5.

ROOF PRESSURE (PSF): AREA, NEGATIVE ZONE 1, NEGATIVE ZONE 2, NEGATIVE ZONE 3, POSITIVE (ALL).

SEISMIC LOAD: SEISMIC DESIGN IS IN ACCORD WITH IBC, RISK CATEGORY PER TABLE 1604.5 IS CATEGORY III.

ANALYSIS PROCEDURE: EQUIVALENT LATERAL LOAD PROCEDURE. DESIGN BASE SHEAR: V = 0.053W (ORDINARY REINFORCED MASONRY SHEAR WALLS).

LATERAL LOAD RESISTANCE SYSTEM: LATERAL LOAD SYSTEM CONSISTS OF ROOF DIAPHRAGMS TRANSFERRING LATERAL LOADS TO MASONRY SHEAR WALLS.

FOUNDATIONS: FOUNDATION DESIGN IS BASED ON GEOTECHNICAL INVESTIGATION PERFORMED BY OLSSON JOB NO. 020-1769 DATED 01/19/2020.

MINIMUM FROST DEPTH = 3'-6". NET ALLOWABLE SOIL BEARING PRESSURE (ON NATIVE SOIL) = 2,000 PSF.

SUBGRADE SHALL BE PREPARED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. CONTRACTOR SHALL READ AND FAMILIARIZE HIMSELF WITH THE GEOTECHNICAL REPORT.

BACKFILLING AGAINST FOUNDATION WALLS WHERE GRADE IS PRESENT ON BOTH SIDES SHALL BE PERFORMED SUCH THAT THE DIFFERENCE IN SOIL HEIGHT ON EACH SIDE DOES NOT EXCEED 2 FEET.

TEMPORARY FROST PROTECTION SHALL BE PROVIDED DURING COLD WEATHER FOR ALL FOUNDATIONS.

CONTRACTOR SHALL PROVIDE FOR PROPER DETERIORATION OF ALL EXCAVATIONS.

HELICAL PIERS: HELICAL PIER DESIGN SHALL BE BASED ON THE RECOMMENDATION OF THE GEOTECHNICAL INVESTIGATION PERFORMED BY OLSSON, JOB NO. 020-1769, DATED JULY 9, 2020.

HELICAL PIERS SHALL BE INSTALLED ALONG EXISTING BUILDING FOUNDATIONS SUPPORTING THE NEW AND EXISTING BUILDING LOADS AS SHOWN ON PLANS AND DETAILS.

HELICAL PIERS SHALL HAVE A MINIMUM ALLOWABLE COMPRESSION CAPACITY EQUAL TO THE WORKING LOADS SHOWN ON PLANS WITH A FACTOR OF SAFETY OF 2.0. DEFLECTION OF PIERS SHOULD NOT EXCEED 1/2".

HELICAL PIERS SHOULD HAVE A MINIMUM TYP DEPTH OF 30' BELOW THE BASE OF THE EXISTING FOOTING.

SUBMIT SHOP DRAWINGS INDICATING ALL PIER INFORMATION, PIER LAYOUTS, AND CAPACITIES OF PIERS SIGNED AND SEAL BY A REGISTERED ENGINEER.

CONCRETE CONSTRUCTION: CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 301 AND ACI 318.

PROVIDE A FORMED CONSTRUCTION KEYWAY BETWEEN ALL HORIZONTAL AND VERTICAL POUR EDGES EXCEPT TOPPING SLABS. PROVIDE WATERSTOP TIPS FOR ALL CONSTRUCTION JOINTS BELOW WATER TABLE AND WHERE INTERIOR SLAB-ON-GRADE IS BELOW EXTERIOR GRADE.

CONCRETE SHALL BE MECHANICALLY CONSOLIDATED IN ACCORD WITH ACI 309.

CONTROL (CONTRACTION OR CONSTRUCTION) JOINTS SHALL BOUND ALL CONCRETE SLABS ON GRADE AS SHOWN ON THE DRAWINGS. WHERE NOT SHOWN ON THE DRAWINGS, CONTROL JOINTS SHALL BE LOCATED SUCH THAT THE ENCLOSED AREA IS RELATIVELY SQUARE AND DOES NOT EXCEED 100 SQUARE FEET.

CONTROL JOINTS SHALL BE LOCATED SUCH THAT THE ENCLOSED AREA IS RELATIVELY SQUARE AND DOES NOT EXCEED 100 SQUARE FEET. PROVIDE TYPICAL CONTROL JOINT LAYOUT DETAIL ON SHEET S3.1. GENERAL CONTRACTOR SHALL SUBMIT CONTROL JOINT SHOP DRAWINGS FOR APPROVAL. KEVED JOINTS NEED ONLY OCCUR AT CONSTRUCTION JOINTS. ALL CONSTRUCTION JOINTS MAY BE SAWCUT. DO NOT PROVIDE CONTROL JOINTS IN STRUCTURAL CONCRETE SLABS AND CONCRETE TOPPING UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS.

CONCRETE REINFORCEMENT: REINFORCING STEEL SHALL BE ASTM A615, GRADE 60. REINFORCING STEEL TO BE WELDED SHALL BE ASTM A706, GRADE 60.

CONCRETE COVER REQUIREMENTS FOR CAST-IN-PLACE, NON-PRESTRESSED CONCRETE UNLESS OTHERWISE NOTED ON DETAILS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3". FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 2".

#6 BARS AND LARGER: #5 BARS AND SMALLER: 1 1/2".

#14 AND #18 BARS: #11 BARS AND SMALLER: 1".

REINFORCING BAR SPICES SHALL BE IN ACCORD WITH THE REQUIREMENTS OF ACI 318.4 AND THE REINFORCING SPICE LENGTH TABLE SHOWN ON THE DRAWINGS.

ALL REINFORCING SHALL BE PROPERLY CHAIRED BY THE CONTRACTOR.

LAP ALL WELDED WIRE REINFORCING AT LEAST ONE FULL WIRE SPACING PLUS 2 INCHES.

MECHANICAL COUPLERS SHALL BE TYPE 2 COUPLERS CAPABLE OF SUSTAINING 125% Fy.

IN LIEU OF WELDED WIRE REINFORCEMENT CALLED OUT ON PLANS, CONTRACTOR MAY USE STRUX 3040 SYNTHETIC FIBER REINFORCEMENT.

RESPONSIBLE FOR DETERMINING QUANTITY OF FIBER REINFORCEMENT REQUIRED TO EXCEED TEMPERATURE AND SHRINKAGE CRACKING BY WELDED WIRE REINFORCEMENT.

FIBER REINFORCEMENT SHALL BE NOTED IN THE BATCH TICKETS AND AVAILABLE FOR THE ENGINEER'S REVIEW UPON REQUEST.

FIBER REINFORCEMENT SHALL NOT BE USED TO REPLACE REINFORCEMENT REQUIRED TO RESIST FLEXURAL OR AXIAL TENSION LOADS.

IF CONCRETE MIX WITHOUT FIBERS IS PLACED IN LIEU OF FIBER REINFORCED CONCRETE WHERE FIBER REINFORCED CONCRETE IS SPECIFIED, CONTRACTOR SHALL BEAR ALL COSTS WITH TESTING, REMOVAL, AND REPLACEMENT.

CAST-IN-PLACE CONCRETE: 1. PROPORTION EACH INDIVIDUAL CONCRETE MIX TO HAVE THE FOLLOWING PROPERTIES:

CLASS LOCATION 28 DAY FC MIX TYPE MAX W/C

A FOOTINGS 3,000 PSI NWT 0.50

B INTERIOR SLABS-ON-GRADE 4,000 PSI NWT 0.40

C TOPPING 4,000 PSI NWT 0.45

D EXTERIOR CONCRETE 4,500 PSI NWT 0.45

NWT = NORMAL WEIGHT CONCRETE (UNIT WEIGHT = 145PCF)

CONCRETE CONTAINING SUPERPLASTICIZING ADMIXTURE SHALL HAVE A SLUMP NOT EXCEEDING 3" PRIOR TO ADDING ADMIXTURE AND NOT EXCEEDING 8" AT PLACEMENT.

THE ADDITION OF WATER TO A CONCRETE BATCH WITH INSUFFICIENT SLUMP SHALL NOT BE PERMITTED.

SUBSTITUTION OF FLY ASH FOR PORTLAND CEMENT IN SLABS ON GRADE AND STRUCTURAL SLABS SHALL NOT BE PERMITTED.

CONTINUOUS FOOTINGS: CONTINUOUS FOOTING REINFORCEMENT SHALL CONTINUE THROUGH ISOLATED SPREAD FOOTINGS WHERE THEY OCCUR.

ALL REINFORCING AT INTERSECTIONS SHALL EXTEND TO THE FAR FACE OF THE INTERSECTING FOOTING.

CONSTRUCTION JOINT LOCATIONS SHALL NOT OCCUR WITHIN EXTENTS OF ISOLATED SPREAD FOOTINGS.

CONTINUOUS FOOTINGS SHALL NOT HAVE CONSTRUCTION JOINTS IN A HORIZONTAL PLANE.

SLABS ON GRADE: ALL SLABS ON GRADE SHALL BE CAST ON A 1/4" VAPOR BARRIER (SEE SPECS) INSTALLED PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

CONTRACTOR SHALL READ AND FAMILIARIZE HIMSELF WITH THE GEOTECHNICAL REPORT. IF DISCREPANCIES EXIST BETWEEN PROJECT SPECIFICATIONS AND GEOTECHNICAL REPORT, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN.

ALL SLABS ON STRUCTURAL DRAWINGS REQUIRE REINFORCING. UNLESS OTHERWISE CALLED OUT ON DRAWINGS, REINFORCE SLABS WITH 6X6 W2 1XW2.1 WELDED WIRE REINFORCING.

PIPING AND CONDUIT SHALL NOT BE INSTALLED IN EMBEDDED SLABS OR SLABS ON GRADE. ROUT ALL PIPING AND CONDUIT BELOW STRUCTURE.

GRADE BEAMS: TOP GRADE BEAM REINFORCING SHALL BE SPLICED ONLY AT MIDSPAN AND BOTTOM GRADE BEAM REINFORCING SHALL BE SPLICED ONLY AT SUPPORTS.

TOP GRADE BEAM REINFORCING SHALL TERMINATE AT TEE AND CORNER INTERSECTIONS WITH A STATIONED BEND OR 90 DEGREE TURN. ALL REINFORCING AT INTERSECTIONS SHALL EXTEND TO THE FAR FACE OF THE INTERSECTING GRADE BEAM.

CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION AND TYPICALLY SHALL OCCUR WITHIN THE CENTER THIRD OF THE GRADE BEAM SPAN.

GRADE BEAMS SHALL NOT HAVE CONSTRUCTION JOINTS IN A HORIZONTAL PLANE.

SIDES OF GRADE BEAMS SHALL BE FORMED. IF EXCAVATIONS ARE MADE NEATLY AND GRADE BEAM WIDTHS ARE INCREASED A MINIMUM OF TWO INCHES, SIDES MAY BE CAST AGAINST THE EARTH OUT.

STRUCTURAL PRECAST CONCRETE: FABRICATOR SHALL BE AN 'APPROVED FABRICATOR' IN ACCORD WITH IBC SECTION 1704.2, REGISTERED AND APPROVED BY THE LOCAL BUILDING DEPARTMENT.

PROVIDE UNITS AS SHOWN ON THE DRAWINGS. MINIMUM 28 DAY CONCRETE COMPRESSIVE STRENGTH SHALL NOT BE LESS THAN 5,000 PSI.

FIELD BOLTING INSTALLATION SHALL BE INSPECTED IN ACCORD WITH THE BUILDING CODE AND THE AISC MANUAL.

A QUALIFIED ENGINEER REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED SHALL DESIGN THE UNITS.

PRECAST SUPPLIER IS RESPONSIBLE FOR DESIGN, DETAILING AND FURNISHING OF ALL HEADERS, FOUR STRIPS, UNIT LAYOUT, OPENINGS THROUGH FLOOR, ETC., AS NECESSARY TO PROVIDE FOR A COMPLETE INSTALLATION.

UNITS SHALL HAVE ONE HOUR UNRESTRAINED FIRE RESISTANCE RATING IN ACCORD WITH UL J949 AND SHALL HAVE UL LABELS.

ALL OPENINGS REQUIRING CUTTING OF STRANDS SHALL BE BY UNIT MANUFACTURER. COORDINATE WITH MECHANICAL, ELECTRICAL AND OTHER TRADES INVOLVED.

DEFLECTION OF PRECAST ROOF OR FLOOR FRAMING MEMBERS NOT SUPPORTING MASONRY WALLS ABOVE SHALL NOT EXCEED SPAN/300 FOR LIVE LOAD ONLY.

PRECAST DESIGN SHALL COMPLY WITH ALL ICC-508 & FEMA 361 REQUIREMENTS.

PRECAST SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE LICENSED ENGINEER RESPONSIBLE FOR THEIR PREPARATION.

COORDINATE WITH ALL OTHER TRADES WHICH PRECAST CONCRETE INTERACTS.

UNLESS OTHERWISE NOTED ON PLAN OR DETAIL, PROVIDE 1 #4 IN TOPPING SLAB EACH SIDE OF OPENINGS EXCEEDING 12 INCHES IN SIZE.

ALL TOPPING SLABS SHALL BE REINFORCED WITH 6X6 W2 9XW2.9 WELDED WIRE REINFORCEMENT.

REINFORCEMENT UNLESS OTHERWISE NOTED. WELDED WIRE FABRIC IN THE TOPPING SHALL BE FLAT SHEETS CENTERED IN THE TOPPING UNLESS OTHERWISE NOTED TO MAINTAIN ITS CORRECT LOCATION.

CONCRETE MASONRY UNITS (CMU): THE MINIMUM 28-DAY COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY UNITS SHALL BE 2,000 PSI ON THE NET AREA.

MORTAR SHALL BE TYPE 'S' FOR CONCRETE MASONRY UNITS IN ACCORDANCE WITH THIS 602 TABLE 9.1.1, MORTAR PROPORTIONS, USING GEMENT LIME OR MORTAR CEMENT.

MINIMUM 28-DAY COMPRESSIVE STRENGTH OF GROUT SHALL BE THE GREATER OF 2,000 PSI OR THE COMPRESSIVE STRENGTH OF THE MASONRY UNITS.

HORIZONTAL JOINT REINFORCING SHALL BE STANDARD LADDER TYPE, GALVANIZED, AT 16-INCHES ON CENTER.

MINIMUM BOND BEAM REINFORCING SHALL BE (2) #5 IN 4" AND 8" WIDE BOND BEAMS AND 2 #6 IN 12" WIDE BOND BEAMS.

SPlice LENGTHS FOR MASONRY REINFORCEMENT SHALL BE IN 72 TIMES THE REINFORCING BAR DIAMETER.

PROVIDE BOND BEAMS AT TOP OF ALL WALLS, AT ROOFS, STRUCTURAL FLOORS, AND WHERE SHOWN ON THE DRAWINGS.

REINFORCING SHALL BE HELD IN PLACE PRIOR TO GROUTING WITH AT LEAST TWO WIRE POSITIONERS PER POUR AND AT REINFORCING SPLICES.

VERTICAL REINFORCING SHALL BE AS FOLLOWS FOR ALL WALLS SUPPORTED ON FOUNDATIONS OR THICKENED SLABS.

PROVIDE ADDITIONAL VERTICAL REINFORCING IN ALL VERTICAL REINFORCED WALLS NOTED ABOVE.

EXTERIOR WALLS: PROVIDE EDGE STIFFENER REINFORCEMENT ON EACH SIDE OF OPENINGS IN SECOND FLOOR VERTICAL CELL.

VERTICAL REINFORCING REQUIRED BY THESE NOTES OR SHOWN ON THE FOUNDATION PLANS SHALL EXTEND FROM FOUNDATION TO TOP OF WALL UNLESS OTHERWISE NOTED.

MASONRY CONTRACTOR SHALL COORDINATE WITH THE MECHANICAL AND ELECTRICAL CONTRACTORS AND BUILD IN OPENINGS FOR ELECTRICAL PANELS, CONDUITS, DUCTWORK, PIPING, FIRE EXTINGUISHER CABINETS, ETC.

CONDUIT AND PIPES SHALL NOT BE RUN IN GROUTED CELLS OF MASONRY UNLESS APPROVED BY ENGINEER.

ALL MASONRY BELOW HIGHEST ADJACENT GRADE SHALL BE GROUTED SOLID.

GROUT SHALL BE MECHANICALLY CONSOLIDATED IN A MANNER TO FILL THE GROUT SPACE AND RECONSOLIDATED IN ACCORD WITH THE INTERNATIONAL BUILDING CODE.

TESTING LABORATORY: TESTING LABORATORY, IN ACCORD WITH IBC REQUIREMENTS, SHALL INSPECT REINFORCEMENT PLACEMENT, GROUT SPACES, AND GROUTING OPERATION.

SPACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 20'. REGULARNESS OF JOINT LAYOUT SHOWN ON ARCHITECTURAL DRAWINGS.

SUBMIT SHOP DRAWINGS WITH PLANS AND ELEVATIONS CLEARLY INDICATING REBAR SIZE, SPACING, LAP LENGTHS, LITELS, JAMBS, CONTROL JOINT LOCATIONS, FOOTING, SLAB, AND ROOF ELEVATIONS.

ALL CMU WALL OPENINGS REQUIRE LITELS AS DEFINED IN THE 'TYPICAL MASONRY LITEL DETAIL' SCHEDULE 'E'.

REFER TO TYPICAL DETAILS FOR MASONRY DETAILS AND REQUIREMENTS NOT SHOWN IN SECTIONS OR PLANS.

AT STEEL LITELS 16 INCHES AND DEEPER, PROVIDE ADJUSTABLE MASONRY ANCHORS AT 8 INCHES ON CENTER VERTICALLY AND 24 INCHES ON CENTER HORIZONTALLY.

STRUCTURAL STEEL: FABRICATOR SHALL BE AISC CERTIFIED AND AN 'APPROVED FABRICATOR' IN ACCORDANCE WITH IBC SECTION 1704.2.1.

STEEL TUBE SHALL MEET ASTM A500, GRADE B.

STEEL PIPE SHALL MEET ASTM A53, TYPE E OR S.

BOLTS AT STEEL TO STEEL CONNECTIONS SHALL BE 3/4-INCH DIAMETER, ASTM A325-N, AND TIGHTENED TO THE SNUG TIGHT CONDITION AS DEFINED BY AISC UNLESS OTHERWISE NOTED.

ANCHOR BOLTS IN CONCRETE OR MASONRY SHALL BE 3/4-INCH DIAMETER ASTM F1554 GRADE 36, UNLESS NOTED OTHERWISE.

FIELD BOLTING INSTALLATION SHALL BE INSPECTED IN ACCORD WITH THE BUILDING CODE AND THE AISC MANUAL.

ALL WELDING SHALL CONFORM TO THE PROVISIONS OF THE AMERICAN WELDING SOCIETY CODE AWS D1.1.

THE TESTING LABORATORY SHALL VISUALLY INSPECT ALL FIELD WELDING.

ALL BOLTS (HIGH STRENGTH, ANCHOR BOLTS, EXPANSION BOLTS, ADHESIVE ANCHORS, ETC) SHALL BE INSTALLED WITH STEEL WASHERS.

ALL WELDS SHOWN ON THE DRAWINGS SHALL BE SHOP WELDS UNLESS NOTED OTHERWISE.

CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF THE BUILDING SYSTEM AT ALL TIMES DURING THE ERECTION PROCESS.

COAT STEEL BELOW GRADE WITH COLD-APPLIED ASPHALT EMULSION PER ASTM D1187.

COORDINATE WITH ALL OTHER TRADES WHICH STEEL INTERACTS.

ANCHOR BOLTS IN CONCRETE OR MASONRY SHALL BE 3/4-INCH DIAMETER ASTM F1554 GRADE 36, UNLESS NOTED OTHERWISE.

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THE TESTING LABORATORY SHALL VISUALLY INSPECT ALL FIELD WELDING.

ALL BOLTS (HIGH STRENGTH, ANCHOR BOLTS, EXPANSION BOLTS, ADHESIVE ANCHORS, ETC) SHALL BE INSTALLED WITH STEEL WASHERS.

ALL WELDS SHOWN ON THE DRAWINGS SHALL BE SHOP WELDS UNLESS NOTED OTHERWISE.

STEEL DECK: STEEL DECK AND ACCESSORIES SHALL BE FROM STEEL CONFORMING TO ASTM A1008 OR ASTM A563.

ROOF DECK SHALL HAVE A MINIMUM YIELD STRENGTH, Fy >= 33 KSI.

ALL STEEL DECK SHALL HAVE ONE COAT OF MANUFACTURER'S STANDARD PRIMER PAINT, UNLESS OTHERWISE NOTED.

DECK WELDING SHALL COMPLY WITH THE BUILDING CODE AND AWS D1.1 USING E60XX ELECTRODES.

ROOF DECK SHALL BE ATTACHED TO SUPPORTING STRUCTURAL MEMBERS AS SHOWN ON DRAWINGS.

PROVIDE 2 INCHES MINIMUM BEARING AT DECK SUPPORTS.

ALL DECK SHALL BE CONTINUOUS OVER 3 SPANS UNLESS OTHERWISE NOTED.

DECK SUPPLIER SHALL VERIFY THAT THE DECK SUPPLIED MEETS OR EXCEEDS THE REQUIRED CLEAR SPANS FOR THE ACTUAL PROJECT.

NO DIRECT ATTACHMENT TO UNDERSIDE OF METAL ROOF DECK IS ALLOWED UNLESS INDICATED ON THE STRUCTURAL DRAWINGS.

MANUFACTURER SHALL SUBMIT CALCULATIONS AND DRAWINGS SEALED BY A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.

LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO SPAN/360 AT SIMPLE SPAN ROOF MEMBERS.

JOIST CANTILEVERS SHALL BE DESIGNED FOR ALL APPLICABLE DEAD, LIVE, AND WIND LOADS.

FABRICATOR SHALL BE AN 'APPROVED FABRICATOR' IN ACCORD WITH IBC SECTION 1704.2.1.

ALL STEEL SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORD WITH IBC SECTION 2207.

SIZE, TYPE AND SPACING OF JOIST BRIDGING TO BE IN ACCORD WITH STEEL JOIST INSTITUTE RECOMMENDATIONS.

DESIGN JOISTS AND BRIDGING TO RESIST A NET UPLIFT PER BUILDING CODE PRESCRIBED LOAD COMBINATIONS.

JOIST JOIST DEPTH SHALL BE 2 1/2" AT K SERIES JOIST AND 5" LH SERIES JOISTS UNLESS OTHERWISE NOTED.

MANUFACTURER SHALL SUBMIT CALCULATIONS AND DRAWINGS SEALED BY A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED.

LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO SPAN/360 AT SIMPLE SPAN ROOF MEMBERS.

JOIST CANTILEVERS SHALL BE DESIGNED FOR ALL APPLICABLE DEAD, LIVE, AND WIND LOADS.

DESIGN JOISTS ACCORDING TO THE STANDARD SPECIFICATION FOR STEEL JOISTS.

ALL JOISTS SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE APPLICABLE U.L. LISTINGS PER THE ARCHITECTURAL DRAWINGS.

PROVIDE SLOPED AND/OR SLOPED AND SKEWED BEARING SEATS AS REQUIRED FOR ROOF SLOPE.

CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW PRIOR TO FABRICATION.

REFER TO SNOW DRIFT DIAGRAM AND/OR LOADING PLAN FOR SNOW DRIFT LOADS.

CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW PRIOR TO FABRICATION.

COPIES OF THE CONTRACT DOCUMENTS SHALL NOT BE SUBMITTED AS SHOP DRAWINGS.

SHOP DRAWINGS SHALL BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR OR CONSTRUCTION MANAGER.

DEFERRED SUBMITTALS: THE FOLLOWING ARE DEFERRED SUBMITTAL ITEMS:

STEEL JOISTS, HELICAL PIERS.

DEFERRED SUBMITTAL CALCULATIONS AND/OR SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE ENGINEER RESPONSIBLE FOR THEIR PREPARATION.

CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AND SHALL CONTACT THE ENGINEER OF RECORD IF ANY DISCREPANCIES ARE FOUND BEFORE PROCEEDING.

EXCAVATION UNDER OR NEAR EXISTING FOUNDATIONS, WHICH DISTURBS THE COMPACTED SOIL BENEATH THE FOOTINGS, WILL NOT BE PERMITTED.

THE SEQUENCE OF CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE RESPONSIBLE FOR PROVIDING ALL TEMPORARY SHORING, BRACING, AND OTHER SUPPORTS AS NEEDED TO SAFELY RESIST ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED.

DEMOLITION: DEMOLITION OF EXISTING STRUCTURE TO BE REMOVED SHALL BE PERFORMED BY THE CONTRACTOR USING MEANS NECESSARY TO PREVENT DAMAGE TO THE EXISTING STRUCTURE.

REPAIR AND SUBMIT THE REPAIR DETAIL WITH CALCULATIONS TO THE ENGINEER OF RECORD FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.

ALL A109 WELDED THREADED STUDS AS MANUFACTURED BY NELSON STEEL WELDING, INC SHALL BE INSTALLED WITH WELD GUN IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS.

POST-INSTALLED ANCHORS: ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND ICC EVALUATION REPORTS.

CHANGE ORDER

Date: August 4, 2021
Comm. Category: 2021031 /
Job No. PR332521

Client Name: Boyd Jones Construction	Change Order #: 001
Client Contact: Mitch Broekemeier	Project Name: Arlington Public Schools
Client Phone: 402.550.1786	Project Location: 705 N 9th Street
Client Email: mbroekemeier@boydjones.biz	Arlington, NE 68002

Proposal: Described below are proposed products/services to be added to the customer's original agreement ("agreement") with Thrasher, Inc.

(21) HP288 Retrofit Helical Piers

An additional charge of \$210 per 7-foot section will be added if HP288 helical piers must be installed to a depth more than (30) feet in order to reach bearing strata.

(6) HP287 New Construction Helical Piles

An additional charge of \$196 per 7-foot section and \$140 per 5-foot section will be added if HP287 helical piles must be installed to a depth more than (30) feet in order to reach bearing strata.

Current Change Order Amount	\$	23,744.00
Original Contract Price:	\$	50,000.00
Previous Change Order Amount:		
Revised Contract Amount:	\$	73,744.00

 Todd Royal - Thrasher, Inc.

 Project Manager

 Date

I authorize and instruct Thrasher, Inc. to add the above products/services to the work specified in the Agreement. I understand and agree that by signing below, this proposal is incorporated in to the terms of the Agreement. If I am not the original signer of the Agreement, I certify that I have authority from the original signer to sign this proposal and authorize the work specified herein.

 Mitch Broekemeier

 Title

 Date

Arlington Public School
 HP288 Retrofit Helical Pier Depths
 \$210 per 7' section for piles going over 30'

HP287 New Construction Helical Pile Depths
 \$196 per 7' section for piles going over 30'
 \$140 per 5' section for piles going over 30'

Pile	Helical Model	Estimated Depth (ft)	Installed Depth (ft)	Difference (ft)	Charge
1	HP288	30	42	12	\$420
2	HP288	30	63	33	\$1,050
3	HP288	30	63	33	\$1,050
4	HP288	30	42	12	\$420
5	HP288	30	42	12	\$420
6	HP288	30	42	12	\$420
7	HP288	30	42	12	\$420
8	HP288	30	42	12	\$420
9	HP288	30	42	12	\$420
10	HP288	30	70	40	\$1,260
11	HP288	30	70	40	\$1,260
12	HP288	30	70	40	\$1,260
13	HP288	30	70	40	\$1,260
14	HP288	30	84	54	\$1,680
15	HP288	30	70	40	\$1,260
16	HP288	30	84	54	\$1,680
17	HP288	30	70	40	\$1,260
18	HP288	30	70	40	\$1,260
19	HP288	30	70	40	\$1,260
20	HP288	30	70	40	\$1,260
21	HP288	30	70	40	\$1,260
22	HP287	30	47	17	\$532
23	HP287	30	42	12	\$420
24	HP287	30	50	20	\$616
25	HP287	30	42	12	\$392
26	HP287	30	42	12	\$392
27	HP287	30	42	12	\$392
TOTAL		810	1553	743	\$23,744

